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GENERAL NOTES

GENERAL

1. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF ANY DISCREPANCY.
2. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE GEORGIA STATE MINIMUM STANDARD BUILDING CODE, BASED ON THE 2006 INTERNATIONAL BUILDING CODE WITH GEORGIA STATE AMENDMENTS.
3. MEANS AND METHODS OF CONSTRUCTION ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
4. THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
5. THE CONTRACTOR SHALL COORDINATE THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL WORKS WITH THE STRUCTURAL DRAWINGS. THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF ANY DISCREPANCIES OR OMISSIONS.
6. THE CONTRACTOR SHALL VERIFY ALL FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT WEIGHTS, FLOOR AND/OR ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. WEIGHTS OF ROOF SUPPORTED EQUIPMENT CONSIDERED IN THE DESIGN ARE INDICATED ON THE DRAWINGS. NOTIFY OWNER'S REPRESENTATIVE IF ACTUAL WEIGHT IS GREATER THAN THE LOAD INDICATED.
8. THICKENED SLABS, DEPRESSED SLABS, TRENCHES, AND UTILITY PENETRATIONS ARE SCHEMATICALLY INDICATED ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL COORDINATE LOCATIONS AND ANY ADDITIONAL REQUIREMENTS WITH THE FINAL ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS AND WITH THE SPECIFIC REQUIREMENTS OF VENDOR DRAWINGS FOR SPECIALTY EQUIPMENT. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER WHERE DISCREPANCIES EXIST BETWEEN THIS INFORMATION AND THE STRUCTURAL DRAWINGS PRIOR TO STARTING WORK.
9. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
10. THE STRUCTURE WILL BE UNSTABLE UNTIL ALL STEEL MEMBERS ARE ERECTED, CONNECTIONS ARE COMPLETELY BOLTED AND/OR WELDED AND INSPECTED, AND THE FLOOR AND ROOF DECK ATTACHED TO THE TILT-UP CONCRETE WALLS AND THE CONCRETE SLAB PLACED AND CURED. UNTIL SUCH TIME, TEMPORARY BRACING IS REQUIRED AND MUST REMAIN IN PLACE.
11. SHOULD ANY OF THE INSTRUCTIONS SHOWN ON THE DRAWINGS CONFLICT WITH THE GENERAL STRUCTURAL NOTES, THE SPECIFICATIONS, OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.
11. ARCHITECTURAL BACKGROUNDS ARE SHOWN FOR REFERENCE ONLY. FINAL STRUCTURAL DOCUMENTS SHOULD BE COORDINATED WITH ARCHITECTURAL CONSTRUCTION DOCUMENTS.

FOUNDATION

1 (revision)

1. THE GEOTECHNICAL REPORT DATED NOV. 23, 2010 AND THE SUPPLEMENTAL GEOTECHNICAL REPORT DATED JANUARY 6, 2011 PROVIDED BY GEO-HYDRO ENGINEERS WERE USED FOR THE DESIGN OF THE FOUNDATIONS. SITE PREPARATION, GRADING, AND SOIL REMEDIATION SHALL BE IN ACCORDANCE TO THE RECOMMENDATIONS PRESENTED IN THIS DOCUMENT.

   SITE PREPARATION

   A. ALL EXISTING STRUCTURES, PAVEMENTS, VEGETATION, ROOTS, AND TOPSOIL SHALL BE COMPLETELY REMOVED FROM WITHIN THE BUILDING EXTENTS. EXISTING STRUCTURES, FOUNDATIONS, AND UNDERGROUND UTILITIES MUST BE THOROUGHLY REMOVED. ALL EXCAVATIONS RESULTING FROM DEMOLITION SHALL BE BACKFILLED WITH STRUCTURAL FILL.
   B. AREAS TO RECEIVE STRUCTURAL FILL SHALL BE PROOFROLLED PRIOR TO PLACEMENT. SEE SPECIFICATION SECTION 02220 AND THE GEOTECHNICAL REPORT FOR RECOMMENDATIONS.
   C. MATERIAL FOR USE AS STRUCTURAL FILL SHOULD BE FREE OF ORGANIC DEBRIS, WASTE CONSTRUCTION DEBRIS, AND OTHER DELETERIOUS MATERIALS. THE MATERIAL SHOULD NOT CONTAIN ROCKS HAVING A DIAMETER OVER 4 INCHES. SUITABLE SOILS FOR USE AS STRUCTURAL FILL ARE SM, SC, ML, AND CL SOILS AS CLASSIFIED BY THE USCS. SEE SPECIFICATION SECTION 02220 AND THE GEOTECHNICAL REPORT FOR STRUCTURAL FILL PLACEMENT.
   D. THE GEOTECHNICAL ENGINEER SHALL PERFORM LABORATORY PROCTOR COMPACTION TESTS AND CLASSIFICATION TESTS ON REPRESENTATIVE SAMPLES OBTAINED FROM THE PROPOSED BORROW MATERIAL TO DETERMINE ACCEPTABILITY PRIOR TO USE.
2. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS THAT VARY FROM THOSE ASSUMED FOR DESIGN.

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3. FOUNDATIONS SHALL CONSIST OF SPREAD FOOTINGS BEARING ON SOIL CAPABLE OF SUPPORTING A MINIMUM OF 3,000 PSF. FOUNDATIONS BEARING ON WEATHERED ROCK OR ON ROCK SHALL BE OVEREXCAVATED FOR A MINIMUM 12 CUSHION OF COMPACTED GRADED AGGREGATE BASE AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER.
4. CONTRACTOR SHALL VERIFY BY AN INDEPENDENT TESTING LABORATORY THAT THE ASSUMED BEARING CAPACITY IS OBTAINED.
5. ALL SOILS SURROUNDING AND UNDER ALL FOOTINGS SHALL BE PROTECTED FROM FREEZING AND FROST ACTION DURING THE COURSE OF CONSTRUCTION.

STEEL JOISTS

1. OPEN WEB STEEL JOISTS, AND JOIST GIRDERS, SHALL BE DESIGNED, FABRICATED, AND ERECTED ACCORDING TO THE AISC, SJI, AND OSHA SPECIFICATIONS.
2. EXTEND ALL JOISTS 1" PAST CENTERLINE OF SUPPORTING MEMBERS WHERE POSSIBLE.
3. ANCHOR ALL STEEL JOIST BRIDGING LINES TO INTERSECTING CONCRETE WALLS, MASONRY WALLS, AND/OR STEEL BEAMS WITH LATERAL ANCHORS UNLESS OTHERWISE SHOWN.
4. ADJACENT JOISTS OF THE SAME DEPTH SHALL HAVE WEB MEMBERS IN LINE WITH EACH OTHER.
5. JOIST MANUFACTURER SHALL DESIGN SPECIAL JOISTS, DESIGNATED "SP" FOR LOAD CRITERIA SHOWN ON DRAWINGS. LOADS SHOWN ON DRAWINGS ARE UNFACTORED, SERVICE LEVEL LOADS. JOIST MANUFACTURER SHALL COORDINATE AND VERIFY WEIGHT, SIZE, AND LOCATION OF EQUIPMENT WITH GENERAL AND MECHANICAL CONTRACTORS.

METAL DECK:

1. STEEL DECK SHALL CONFORM TO THE SPECIFICATIONS OF THE STEEL DECK INSTITUTE (SDI).
2. METAL DECK SHALL BE FURNISHED IN SHEET LENGTHS SUFFICIENT TO EXTEND OVER FOUR SUPPORTS (3 SPANS) WHEREVER POSSIBLE. THE DECK SUPPLIER SHALL ADJUST THE THICKNESS OR GAGE OF THE DECK AT LOCATIONS WHERE SINGLE OR DOUBLE SPANS ARE PROVIDED. DECK SHALL BE DESIGNED TO PROVIDE EQUIVALENT OR GREATER LOAD CAPACITY AS THE SPECIFIED DECK SUPPORTED OVER THREE CONTINUOUS SPANS.
3. FORM DECK AND ROOF DECK TO BE LAPPED 2" MIN.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING DESIGNATIONS:

ALL COLUMNS, BEAMS, AND GIRDERS UNLESS OTHERWISE NOTED.	ASTM A992
PLATES, RODS, ANGLES CHANNELS AND BARS UNLESS OTHERWISE NOTED	ASTM A36
HSS SECTIONS GRADE B	ASTM A500
STRUCTURAL STEEL PIPES TYPE E OR GRADE B.	ASTM A53

2. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE AISC "LRFD SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"
3. CONNECTIONS MAY BE WELDED OR BOLTED. BOLTED CONNECTIONS SHALL BE BEARING TYPE WITH MINIMUM 3/4" DIA. A-325 TENSION CONTROL BOLTS. FIELD WELDING SHALL BE BY CERTIFIED WELDERS IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS).
4. STANDARD SHEAR CONNECTIONS SHALL BE SELECTED BY THE FABRICATOR AND CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION. CONNECTIONS SHALL BE PROVIDED TO CONFORM TO THE REQUIREMENTS OF TYPE 2 CONSTRUCTION UNLESS NOTED OTHERWISE. FACTORED BEAM OR GIRDER END REACTIONS ARE INDICATED ON PLAN FOR CONNECTION SELECTION. MINIMUM CONNECTION SHALL CONSIST OF TWO ROWS OF BOLTS. WHERE NO VALUE IS SHOWN, REACTION IS LESS THAN OR EQUAL TO 20 KIPS FACTORED.
5. ANCHOR RODS SHALL BE PROVIDED WITH HEADS AND HAVE THE MINIMUM PROPERTIES OF ASTM F1554 GR 36 WELDABLE, UNLESS NOTED OTHERWISE.
6. WHERE STRUCTURAL STEEL IS EXPOSED TO EARTH OR WEATHER, PROVIDE A PROTECTIVE COAT CONSISTING OF BITUMINOUS MASTIC OR SIMILAR.

CONCRETE

1. CONCRETE WORK SHALL CONFORM TO THE ACI 318 AND CRSI STANDARDS.
2. CONCRETE SHALL HAVE THE FOLLOWING 28-DAY COMPRESSIVE STRENGTH UTILIZING TYPE I CEMENT:

FOOTINGS	3,000 PSI (NORMAL WEIGHT)
SLAB-ON-GRADE	3,000 PSI (NORMAL WEIGHT)
CAST-IN-PLACE CONCRETE WALLS	4,000 PSI (NORMAL WEIGHT)
TILT-UP CONCRETE WALLS	4,000 PSI (NORMAL WEIGHT)
ELEVATED SLAB	2,500 PSI (NORMAL WEIGHT)

3. WATER/CEMENT RATIO, FLY ASH CONTENT AND MAXIMUM SLUMPS ARE SPECIFIED IN SECTION 03310 OF THE PROJECT SPECIFICATIONS. SUBSTANTIATING DATA FOR CONCRETE MIX DESIGN ARE ALSO DESCRIBED IN THIS SECTION.
4. PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND/OR ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.
5. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.

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6. SUBMIT THE FOLLOWING TO THE ARCHITECT (STRUCTURAL ENGINEER) FOR APPROVAL: ENGINEERING CALCULATIONS AND CONSTRUCTION DOCUMENTS BARING THE SEALOF A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED FOR ALL TILT-WALL PANELS THAT HAVE BEEN DESIGNED FOR THE FOLLOWING MINIMUM LOADS AS INDICATED IN THESE DRAWINGS:

   SELF WEIGHT DEAD LOAD
   FLOOR DEAD LOADS
   ROOF DEAD LOADS
   FLOOR LIVE LOADS
   ROOF LIVE LOADS
   ROOF SNOW LOADS
   WINDS LOADS
   SEISMIC LOADS
7. TILT-UP CONCRETE WALL CONTRACTOR IS SOLELY RESPONSIBLE FOR COORDINATION OF ALL EMBEDDED ITEMS AND THE TIMELY DELIVERY OF THESE ITEMS TO THE SITE PRIOR TO CASTING OF CONCRETE.
8. THE DESIGN OF ALL PANEL LIFTING ELEMENTS SHALL BE THE RESPONSIBILITY OF THE TILT-UP CONCRETE WALL CONTRACTOR. DESIGN OF ALL TILT-UP WALL LIFTING ELEMENTS SHALL BE PERFORMED BY A LICENSED ENGINEER IN THE STATE OF GEORGIA. TILT-UP WALL SHOP DRAWINGS DETAILING ALL WALL REVEALS, EMBEDDED ITEMS, OPENINGS, ETC. SHALL BE SUBMITTED FOR REVIEW.
9. PROVIDE NON-METALLIC NONSHRINK GROUT WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5,000 PSI.
10. PROVIDE LENTON FORMSAVER AND THREADED REINFORCING WHERE INDICATED. SEE ICC ER-3967.

REINFORCING STEEL

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615 - GRADE 60
2. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706- GRADE 60
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND HAVE A MINIMUM SIDE LAP OF 8".
4. REINFORCEMENT SHALL BE SPLICED ONLY AS SHOWN OR NOTED. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.
5. PROVIDE DOWELS IN FOOTINGS THE SAME SIZE AND NUMBER AS VERTICAL WALL OR COLUMN REINFORCING. DOWELS SHALL HAVE A MINIMUM STANDARD LAP SPLICE UNLESS OTHERWISE SHOWN.
6. PROVIDE THE FOLLOWING PROTECTIVE COVERING OF REINFORCING BARS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	
ALL REINFORCEMENT SIZES	3" CLEAR
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2" CLEAR
#5 BAR, W31 OR D31 WIRE, AND SMALLER	1-1/2" CLEAR
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS: #14 AND #18 BARS	1-1/2" CLEAR
#11 BAR AND SMALLER	3/4" CLEAR
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, AND SPIRAL	1-1/2" CLEAR

7. ALL REINFORCING BARS TO BE SPLICED WITH THE FOLLOWING STANDARD LAP LENGTHS:

BAR NO.	BOTTOM BAR LENGTH (INCHES)	TOP BAR * LENGTH (INCHES)
#3	19	25
#4	25	33
#5	31	41
#6	37	48
#7	54	70
#8	62	81
#9	70	91
#10	78	102
#11	85	111

*TOP BAR IS HORIZONTAL REINFORCEMENT IN FLEXURAL MEMBERS SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE SPLICE.

8. REINFORCING BARS MARKED CONTINUOUS TO BE SPLICED WITH THE STANDARD LAP LENGTH.
9. PROVIDE CORNER BARS TO MATCH ALL LONGITUDINAL REINFORCEMENT AT ALL CORNERS AND INTERSECTIONS OF ALL CONCRETE MEMBERS (FOOTINGS, GRADE BEAMS, WALLS, ETC.) UNLESS OTHERWISE DETAILED.

GRAVITY DESIGN LOADS

1. LIVE LOADS

   OFFICE BUILDINGS:
   50 PSF OFFICES
   80 PSF CORRIDORS ABOVE 1ST FLOOR
   100 PSF LOBBIES AND 1ST FLOOR CORRIDORS
   20 PSF ROOF
2. PARTITIONS 20 PSF
3. SNOW LOADS

   5 PSF GROUND SNOW LOAD WITH APPLICABLE FACTORS

FLAT ROOF SNOW LOAD, P_F	= 5 PSF
SNOW EXPOSURE FACTOR, C_e	= 1.0
SNOW LOAD IMPORTANCE FACTOR, I_S	= 1.0
THERMAL FACTOR, C_t	= 1.0

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4. DEAD LOAD

FLOOR	= 28 PSF
ROOF	= 76 PSF

LATERAL DESIGN LOADS

BUILDING OCCUPANCY CATEGORY = II

1. WIND LOADS

BASIC WIND VELOCITY	= 90 mph (3 SEC GUST)
IMPORTANCE FACTOR, I_W	= 1.0
EXPOSURE CATEGORY	= B

2. SEISMIC LOADS

IMPORTANCE FACTOR, I_E	= 1.0
SPECTRAL RESPONSE ACCELERATION, S_S	= 0.260g
SPECTRAL RESPONSE ACCELERATION, S_1	= 0.092g
SOIL SITE CLASSIFICATION	= C
DESIGN SPECTRAL ACCELERATION, S_{DS}	= 0.208g
DESIGN SPECTRAL ACCELERATION, S_{D1}	= 0.104g
SEISMIC DESIGN CATEGORY	= B
BASIC SEISMIC FORCE RESISTING SYSTEM	= ORDINARY PRECAST SHEAR WALLS
DESIGN BASE SHEAR	= 0.07 x BUILDING WEIGHT
RESPONSE MODIFICATION FACTOR, R	= 3
ANALYSIS PROCEDURE	= EQUIVALENT LATERAL FORCE PROCEDURE

1 (revision)

QUALITY ASSURANCE

1. A STATEMENT OF SPECIAL INSPECTIONS INCLUDING A SCHEDULE OF SPECIAL INSPECTIONS HAS BEEN PREPARED IN COMPLIANCE WITH THE GEORGIA STANDARD BUILDING CODE AND IS A PART OF THESE CONTRACT DOCUMENTS.
2. THE THE STATEMENT OF SPECIAL INSPECTIONS HAS IDENTIFIED FIRM(S) AND/OR INDIVIDUAL(S) RESPONSIBLE FOR THE DOCUMENTATION AND REPORTING OF THE REQUIRED TESTS, INSPECTIONS AND OBSERVATIONS.
3. THE SPECIAL INSPECTIONS SHALL COMPLY WITH THE GEORGIA STANDARD BUILDING CODE AND ANY OTHER REQUIREMENTS OF THE CONTRACT DOCUMENTS.
4. SUBMIT SPECIAL INSPECTION INTERIM REPORTS AT EACH INTERVAL OR STAGE OF CONSTRUCTION IN COMPLIANCE WITH THE GEORGIA STANDARD BUILDING CODE AS REQUIRED BY THE STATMENT OF SPECIAL INSPECTIONS.
5. WRITTEN NOTIFICATION WILL BE PROVIDED BY THE INSPECTOR OF ANY DEVIATIONS FROM THE APPROVED PLANS AND SPECIFICATIONS OR NEW CONSTRUCTION NOT IN COMPLIANCE WITH THE GEORGIA STANDARD BUILDING CODE, WHICH ARE NOT IMMEDIATELY CORRECTED.

POST INSTALLED FASTENERS

1. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD (EOR) PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
2. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE. IF REINFORCING STEEL IS ENCOUNTERED DURING DRILLING, ADJUST ANCHOR LOCATIONS WITHIN 3 INCHES OF SPECIFIED LOCATION AND AT LEAST 1 INCH FROM ANY ABANDONED HOLE. FILL ABANDONED HOLES WITH HIGH-STRENGTH NON-SHRINK GROUT.
3. SPECIAL INSPECTION SHALL BE PROVIDED FOR ALL ADHESIVE AND MECHANICAL ANCHOR INSTALLATIONS AS REQUIRED BY THE STATEMENT OF SPECIAL INSPECTIONS. INDEPENDENT ON-SITE PROOF LOAD TESTING BY THE SPECIAL INSPECTOR SHALL BE PERFORMED AS INDICATED BELOW.

MECHANICAL ANCHORS TESTING REQUIREMENTS

ANCHOR Φ [IN]	MINIMUM EMBEDMENT [IN]	MINIMUM TENSION PROOF LOAD [LB]	MINIMUM TORQUE PROOF LOAD [FT-LB]
3/8	2	2500	25
1/2	2	3000	40
5/8	3⅛	4600	60
3/4	3¾	5600	110

NOTES:
1. ACCEPTANCE CRITERIA FOR MECHANICAL ANCHORS:
   HYDRAULIC RAM METHOD: ANCHORS TESTED WITH A HYDRAULIC RAM (JACK) OR SPRING LOADED DEVICES SHALL MAINTAIN THE TEST LOAD FOR A MINIMUM OF 15 SECONDS AND SHALL EXHIBIT NO DISCERNABLE MOVEMENT DURING THE TENSION TEST, E.G., AS EVIDENCED BY THE LOOSENING OF THE WASHER UNDER THE NUT.

   CALIBRATED TORQUE WRENCH METHOD: ANCHORS TESTED WITH A CALIBRATED TORQUE WRENCH MUST ATTAIN THE SPECIFIED TORQUE WITHIN ½ TURN OF THE NUT.
2. PROOF LOAD TEST 100% OF THE MECHANICAL ANCHORS.
3. CONCRETE STRENGTH BASED ON f'_c = 3000 PSI

ADHESIVE ANCHORS TESTING REQUIREMENTS

THREADED ROD ANCHOR Φ [IN]	MINIMUM EMBEDMENT [IN]	MINIMUM TENSION PROOF LOAD [LB]
3/8	3	5400
1/2	4	9750
5/8	5	15000
3/4	7	23000

NOTES:
1. ACCEPTANCE CRITERIA FOR ADHESIVE ANCHORS:
   HYDRAULIC RAM METHOD: ANCHORS TESTED WITH A HYDRAULIC RAM (JACK) SHALL MAINTAIN THE TEST LOAD FOR A MINIMUM OF 15 SECONDS AND SHALL EXHIBIT NO DISCERNABLE MOVEMENT DURING THE TENSION TEST, E.G., AS EVIDENCED BY THE LOOSENING OF THE WASHER UNDER THE NUT.
2. PROOF LOAD TEST 50% OF THE THREADED ROD ANCHORS.
3. CONCRETE STRENGTH BASED ON f'_c = 3000 PSI.

DEFORMED REINFORCING BAR ANCHOR [IN]	MINIMUM EMBEDMENT [IN]	MINIMUM TENSION PROOF LOAD [LB]
#3	4	5300
#4	5	9600
#5	6	14900
#6	7	21120

NOTES:
1. ACCEPTANCE CRITERIA FOR ADHESIVE ANCHORS:
   HYDRAULIC RAM METHOD: ANCHORS TESTED WITH A HYDRAULIC RAM (JACK) SHALL MAINTAIN THE TEST LOAD FOR A MINIMUM OF 15 SECONDS AND SHALL EXHIBIT NO DISCERNABLE MOVEMENT DURING THE TENSION TEST, E.G., AS EVIDENCED BY THE LOOSENING OF THE WASHER UNDER THE NUT.
2. PROOF LOAD TEST 50% OF THE DEFORMED REINFORCING BAR ANCHORS.
3. CONCRETE STRENGTH BASED ON f'_c = 4000 PSI.

4. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE EOR WITH CALCULATIONS SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE REQUIRED BY THE BUILDING CODE. PRODUCT ICC-ES CODE REPORTS SHALL BE INCLUDED WITH SUBMITTAL PACKAGE.

   A. ADHESIVE ANCHORS SHALL BE HILTI HIT RE500SD CONFORMING TO ICC ESR-2322 OR APPROVED EQUAL.
   B. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ CONFORMING TO ICC ESR-1917 OR APPROVED EQUAL.
   C. POWER DRIVEN FASTENERS (PDF'S) SHALL CONFORM TO ICC ESR-2138 OR APPROVED EQUAL.

LATTA STRUCTURAL ENGINEERS, LLC
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GEORGIA REGISTERED PROFESSIONAL ENGINEER
BRENT E. LATTA
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ATHENS MEDICAL PLAZA I
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REVISION

NO.	DATE	DESCRIPTION
1	02/03/11	REVISION #1
3	05/06/11	REVISION #3
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GENERAL NOTES

S1.0

ISSUE DATE:	JOB NUMBER
12.21.10	08004